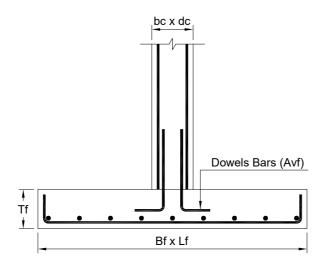


# <u>Design for Transfer of Horizontal Force at Base of Column where The Footing Surface is not Intentionally</u> Roughened as per ACI 318-11 Chapter 12



#### **System**

| Column Width, b <sub>c</sub> =      | 12.0 in |
|-------------------------------------|---------|
| Column Depth, d <sub>c</sub> =      | 12.0 in |
| Footing Width, B <sub>f</sub> =     | 9.0 ft  |
| Footing Length, L <sub>f</sub> =    | 9.0 ft  |
| Footing Thickness, T <sub>f</sub> = | 22.0 in |

#### Load

| Ultimate Horizontal Force at the Base of Column, V <sub>II</sub> = | = 84.0 kips |
|--|-------------|
|--|-------------|

### **Material Properties**

| Concrete Strength, f' <sub>c</sub> =   |   | 4000 psi  |
|--|---|-----------|
| Yield Strength of Reinforcement, f <sub>y</sub> =                            |   | 60000 psi |
| Shear Strength Reduction Factor (According to Cl.9.3.2 of ACI318), $\Phi$ =  |   | 0.75      |
| Modification Factor for Lightweight Concrete, $\lambda$ =                    |   | 1.00      |
| Friction Factor (According to Cl.11.6.4.3 of ACI318), $\mu$ = 0.6* $\lambda$ | = | 0.60      |

#### **Check on Maximum Shear Transfer Permitted**

Nominal Shear Force (According to Cl.11.6.5 of ACI318),

| $\Phi V_{n1} =$   | $\Phi^*(0.2*f_c/1000*b_c*d_c)$                         | = | 86.4 kips |  |  |
|---|--|---|-----------|--|--|
| $\Phi V_{n2} =$   | $\Phi^*(800^*b_c^*d_c)/1000$                           | = | 86.4 kips |  |  |
| Minimum Nominal Shear, $\Phi V_n$ =                               | $MIN(\PhiV_{n1};\PhiV_{n2};)$                          | = | 86.4 kips |  |  |
| Check Validity=   | IF(Vu<ΦV <sub>n</sub> ; "Valid"; "Increase Dimension") | = | Valid     |  |  |
| Required Area of Reinforcement (According to Eq.11-25 of ACI318), |  |   |           |  |  |
|   | Vu*1000  |   | •         |  |  |

$$A_{\text{vf}} = \frac{\text{vu}^*1000}{\Phi^* \text{fy}^* \mu} = 3.11 \text{ in}^2$$

# C

## **Transfer of Horizontal Force at Base of Column**

**ACI 318** 

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| Provided Shear Reinforcement, Bar= | SEL("ACI/Bar"; Bar; )        | = No.8      |
|------------------------------------|------------------------------|-------------|
| Diameter of Bars, Dia=             | TAB("ACI/Bar"; Dia; Bar=Bar) | = 1.0000 in |
| Number of Bars n=                  |                              | 4           |

Provided Area of Reinforcement, 
$$A_s = n^* \frac{\pi^* Dia^2}{4}$$
 = 3.14 in<sup>2</sup>

Check Validity= 
$$IF(A_s>A_{vf}; "Valid"; "Increase RFT")$$
 = Valid

#### Check on Development Length of Tensile Reinforcement with Column

Clear Cover to Center of Bars, c= 3.25 in Center to Center Bar Spacing, S= 4.50 in Factor of, cb= MIN(c+Dia/2; S/2) = 2.25 in (According to CI.12.2.3 of ACI318) Factor of, Ktr= 0.00 (According to CI.12.2.4 of ACI318) Factor of, 
$$\Psi_t$$
= 1.00 (According to CI.12.2.4 of ACI318) Factor of,  $\Psi_e$ = 1.00 (According to CI.12.2.4 of ACI318) Factor of,  $\Psi_e$ = 1.00

#### **Development Length within Column**

Development Length (According to Eq.12-1 of ACI318),

$$L_{d1} = \frac{3}{40} * \frac{fy}{\lambda^* \sqrt{fc}} * \frac{\Psi_t * \Psi_e * \Psi_s}{(cb + Ktr)/Dia} * Dia = 31.6 in$$

## **Development Length within Footing**

Development Length (According to Cl.12.5.2 of ACI318),

$$L_{d2} = \frac{0.02 * \Psi_e * fy}{\lambda^* \sqrt{f'c}} * Dia = 19.0 in$$

#### **Design Summary**

Provided Area of Reinforcement, 
$$A_s$$
 =  $A_s$  = 3.14 in<sup>2</sup>  
Development Length within Column,  $L_{d1}$  =  $L_{d1}$  = 31.6 in  
Development Length within Footing,  $L_{d2}$  =  $L_{d2}$  = 19.0 in